	ERNEST ORLANDO LAWRENCE BERKELEY NATIONAL LABORATORY		BaBar Detector
	ENGINEERING NOTE Mechanical Engineering Department		
WBS#: 1.3.1.1	LBNL Serial #: M7621	DIRC Note #: 74	Page 1 of 21
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First Line: BaBar Detector
Second Line: DIRC PID
Title: Support Structure Bolting and Pinning

MAG

INTRODUCTION

This note summarizes the calculations that were performed to size bolts and pins in the DIRC Support Structure. The assembly is important to the structural integrity of the larger BaBar Detector because it supports the Backward End Plug, the DIRC Standoff Box, and the Drift Chamber. The design of bolted and pinned connections between components in this structure was judged to be of critical importance due to the magnitude of magnetic and seismic loads which it must withstand.

LOAD DESCRIPTION

The analysis utilized loads calculated using an ANSYS, Rev 5.3, finite element model of the DIRC subsystem, "Finite Element Stress Analysis of the DIRC Structure," MCR Associates, Inc. 10/15/96. An analysis was performed to obtain loads on the joints for the static operating condition as well as worst case loads during seismic events. Using nodal force summations over interfaces between components, net axial and shear forces were extracted. In order to guarantee the conservatism of load data, all axial loads were assumed to act in tension on the bolts. Forces were averaged per bolt over each interface zone.

JOINT ANALYSIS

The connections were designed to achieve a total safety factor on static loading of at least three and a total safety factor on seismic loading of at least one, as shown in Table #1. Shear force was resisted by bolts and shear pins in those locations in which an increase in magnetic loading could cause catastrophic failure in slippage between the members. In locations where magnetic loading could not cause catastrophic slip failure, shear forces were resisted by bolts with no shear pins. All joints are designed as slip-critical, rather than bearing-type connections. In other words the joints resist shear through friction between the members rather than by loading the bolts in shear.

The analysis followed the technique prescribed by the *American Institute of Steel Construction (AISC) Manual of Steel Construction*, 9th edition, Part 5, Specification for Structural Steel Buildings - Allowable Stress Design and Plastic Design, and

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Specification for Structural Joints Using ASTM A325 or A490 Bolts. The code could not be adhered to strictly because metric bolts were used in the design. Metric property class 8.8 socket head cap screws and standard hex head screws were used instead of A325 or A490 heavy hex head bolts. The analysis assumes a Class B coating on both faces comprising each bolted interface. According to Appendix A of the AISC Specification for Structural Joints Using ASTM A325 or A490 Bolts, this coating guarantees a slip coefficient of greater than or equal to 0.5. In locations in which clearance holes were made oversized in order to allow for adjustment, the allowable slip load was decreased in accordance with the code.

JOINT SPECIFICATIONS

LBNL Specification number M871 (DIRC Note number 73) describes the requirements for fasteners, joint preparation, and bolt tightening in the DIRC Support Structure. The attached drawings, load data, and calculations show how the size and number of fasteners were determined for each joint. The structure was divided into nineteen zones, shown in Figure #1 and Figure #2, for which separate loads and solutions were determined. In some zones safety factors on static loading were higher or slightly lower than three. In these cases, geometric and mechanical constraints affected the selection of fasteners.

The table below summarizes the results of the bolt and shear pin sizing calculations for each zone.

Table #1: Summary of Bolting and Pinning

Zone	Pin Diam. (mm)	Number of pins	Bolt Diam. (mm)	Number of Bolts	Total Safety Factor for Slippage Under Seismic Loading	Total Safety Factor for Slippage Under Static Loading
1	12	10	12	11	2.90	3.23
2	-	-	-	-	-	-
3	30	7	30	7	6.56	7.12
4	30	7	30	7	2.53	3.04
5			36	11	2.84	3.13
6			36	14	2.54	3.10
7			30	8	1.44	8.74
8			25.4	13	2.35	2.58
9			38.1	18	3.31	5.33
10			38.1	6	10.42	29.29
11	30	4	30	6	3.53	3.54
12			38.1	10	2.95	2.95
13	30	7	30	8	6.98	7.51
14	30	7	30	8	2.85	3.22
A			24	24	22.57	35.58
B			24	60	33.69	33.79
C			24	60	20.90	22.43
D			20	12	1.79	12.20
E			4	6	6.54	6.54

COMMENTS

A major challenge in determining the size and number of fasteners proved to be designing to an American structural engineering code while using metric fasteners. The bolts specified, metric property class 8.8 bolts, have an equivalent or higher yield strength than do ASTM A325 bolts. The minimum yield strength of property class 8.8 bolts is 92,800 psi. The yield strength of ASTM A325 bolts ranges from 81,000 psi to 92,000 psi, depending on the size of the fastener. As a result of the slightly higher yield strength of the metric bolts, the safety factors calculated for the bolts are assumed to be conservative because they are based on a bolt yield strength of 81,000 psi. Metric A325 bolts exist but are extremely difficult to find compared to the readily available property class 8.8 metric bolts.

$$SF_{BOLTS} = \frac{28000 \text{ psi} \left(1 - \frac{0lb}{0.7 \times 7 \times 81000 \text{ psi} \times 1.0956 \text{ in}^2} \right) 1.0956 \text{ in}^2 \times 7}{208846lb} = 1.03$$

Solve for the safety factor for bolt yield under the combination of pre-tension and applied axial load using Equation #4:

EQUATION #4

$$SF_{by} = \frac{S_y}{0.7S_y + \frac{F_A}{N_b A_b}}$$

Solve for the safety factor on bolt yield under seismic loading using Equation #4:

$$SF_{by} = \frac{81000 \text{ psi}}{0.7 \times 81000 \text{ psi} + \frac{3848lb}{7 \times 1.0956 \text{ in}^2}} = 1.42$$

Solve for the safety factor on bolt yield under static loading using Equation #4:

$$SF_{by} = \frac{81000 \text{ psi}}{0.7 \times 81000 \text{ psi} + \frac{0lb}{7 \times 1.0956 \text{ in}^2}} = 1.43$$

Pin Sizing

Solve for the minimum number of shear pins required, N_{MIN} to obtain a safety factor of one on seismic loading (i.e.: stressing the pins to their shear strength) using Equation #5:

EQUATION #5:

$$N_{MIN} = \frac{F_{SHEAR}}{S_y A_b} \quad (\text{rounded up to nearest integer})$$

$$N_{MIN} = \frac{250747lb}{54815 \text{ psi} \times 1.0956 \text{ in}^2} = 4.1753 \rightarrow 5 \text{ shear pins}$$

The number of shear pins selected for this joint, N, is 7. Solve for a safety factor for failure through shear of the pins according to Equation #6:

EQUATION #6:

$$SF_{PINS} = \frac{S_y N A_b}{F_{SHEAR}}$$

Solve for the safety factor for seismic loading according to Equation #6:

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$$SF_{PINS} = \frac{54815 \text{psi} \times 7 \times 1.0956 \text{in}^2}{250747 \text{lb}} = 1.68$$

Solve for the safety factor for static loading according to Equation #6:

$$SF_{PINS} = \frac{54815 \text{psi} \times 7 \times 1.0956 \text{in}^2}{208846 \text{lb}} = 2.01$$

Total Safety Factor for Joint Slippage:

The total safety factor for the joint is calculated by adding the safety factors for bolts and shear pins for each case. This direct addition is valid because the safety factors for the bolts and shear pins were calculated independently.

$$\text{Total Seismic Safety Factor for Slippage} = SF_{BOLTS} + SF_{PINS} = 0.85 + 1.68 = 2.53$$

$$\text{Total Static Safety Factor for Slippage} = SF_{BOLTS} + SF_{PINS} = 1.03 + 2.01 = 3.04$$

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FIGURE #1: BOLTING AND PINNING ZONES

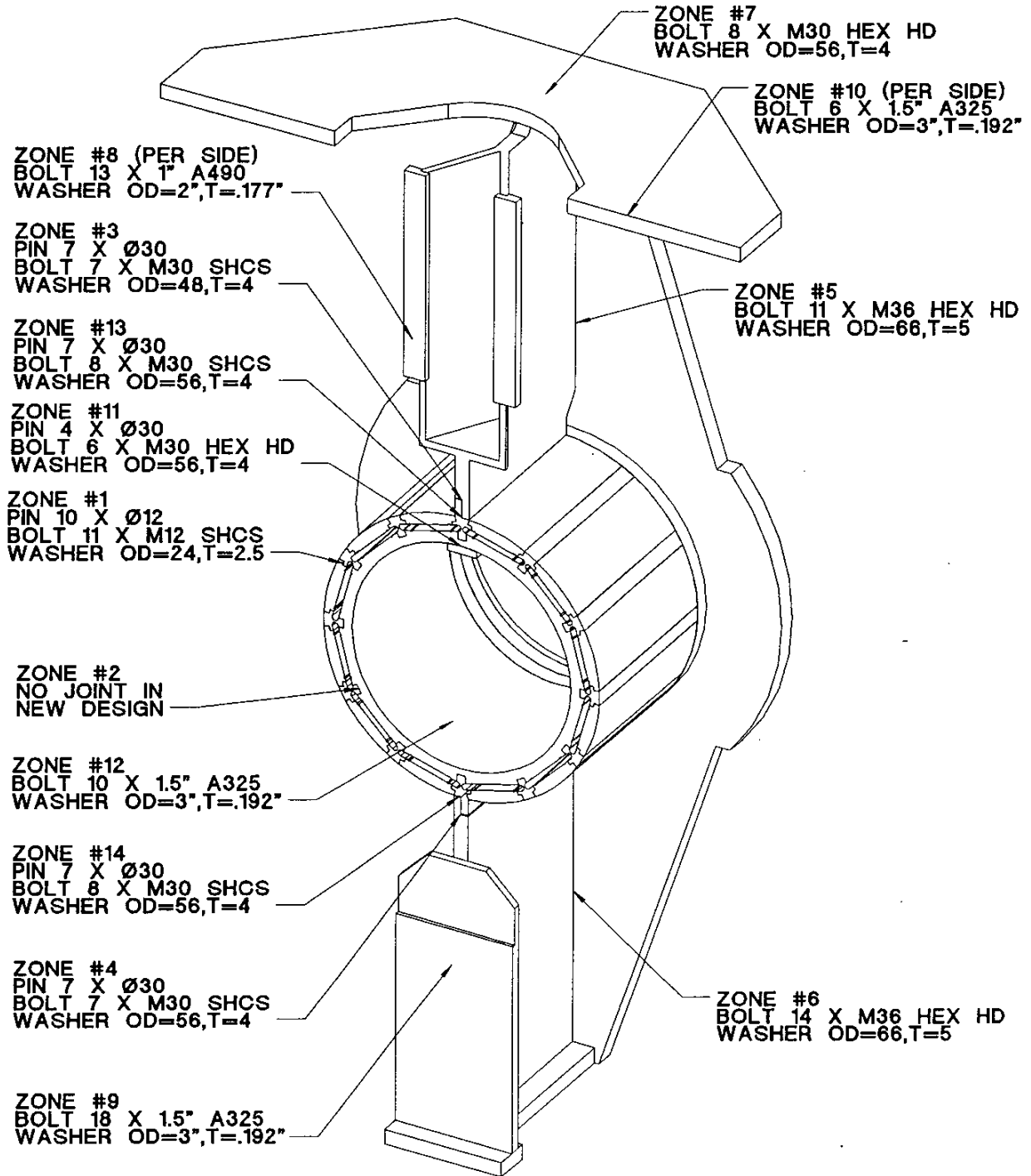


FIGURE #2: INTERNAL BOLTING ZONES

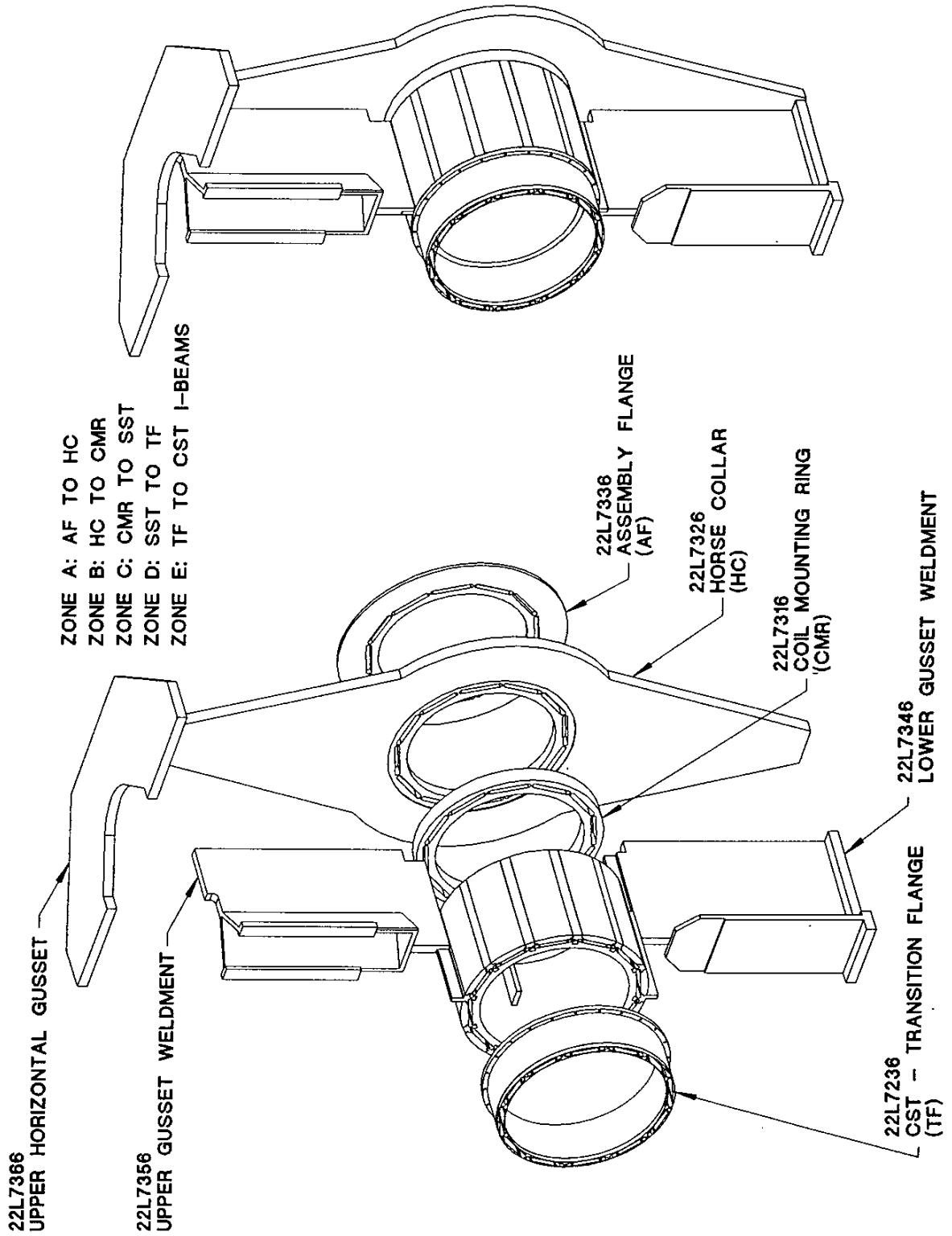


Table #2: Bolt Sizing

Bolt Properties: Assume A325 Bolts
 Source: AISC Manual of Steel Construction, Ninth Edition
 Yield Strength $F_y = 81000$ psi

Bolt Properties: Zone 8: A490 Bolts
 Source: AISC Manual of Steel Construction
 Yield Strength $F_y = 131000$ psi

Assume Class B Contact Surfaces
 Slip Coefficient $\mu_s = 0.5$
 Allowable Slip Load **A325**
 per Unit of Bolt Area = $F_v = 28000$ psi **A490**
 For Oversized Holes = $F_v = 24000$ psi
 Tighten bolts to: **70%** of Yield Strength

Bolts sized to resist shear force with friction

Zone	Equation	Seismic Shear Force (lb) F_{SEISM}	Seismic Axial Force (lb) F_A	Static Shear Force (lb) F_{SHEAR}	Static Axial Force (lb) F_A	Total Area Req. (in ²) A_t	Bolt Diam. (mm) d	Area of Bolt (in ²) A_b	Minimum Number of Bolts Req. N_{MIN}	Chosen Number of Bolts N	Safety Factor Seismic (slippage)		Safety Factor Static (slippage)		Safety Factor Seismic (bolt yield)		Safety Factor Static (bolt yield)	
											Equation #3	Equation #3	Equation #3	Equation #3	Equation #4	Equation #4	Equation #4	Equation #4
1	50837	5237	5237	46153	1739	1.8856	12	0.1753	11	11	1.01	1.15	1.36	1.41	1.36	1.41		
2	109984	5380	5380	69830	25810	4.85622575	-	-	-	-	-	-	-	-	-	-	-	
3	96575	2452	2452	89213	0	3.4498	30	1.0956	4	7	2.21	2.41	1.42	1.43	1.42	1.43		
4	250747	3848	3848	208846	0	8.9126	30	1.0956	9	7	0.85	1.03	1.42	1.43	1.42	1.43		
5	165865	30862	30862	151338	24840	6.3949	36	1.5777	5	11	2.84	3.13	1.39	1.39	1.39	1.39		
6	236890	34606	34606	194772	28368	8.9662	36	1.5777	6	14	2.54	3.10	1.39	1.40	1.39	1.40		
7	167596	7916	7916	27810	4972	6.0513	30	1.0956	6	8	1.44	8.74	1.41	1.41	1.41	1.41		
8	116656	43853	43853	106775	39305	3.0226	25.4	0.7854	4	13	2.35	2.58	2.15	2.16	2.15	2.16		
9	197070	263500	263500	139267	48665	11.5986	38.1	1.7671	7	18	3.31	5.33	1.25	1.39	1.25	1.39		
10	28319	3592	3592	10117	1105	1.0623	38.1	1.7671	1	6	10.42	29.29	1.42	1.43	1.42	1.43		
11	120000	9400	9400	120000	0	4.2389	30	1.0956	4	6	1.53	1.53	1.53	1.43	1.53	1.43		
12	152000	9400	9400	152000	94400	7.0255	38.1	1.7671	4	10	2.95	2.95	1.31	1.31	1.31	1.31		
13	93783	23184	23184	87498	17408	3.7169	30	1.0956	4	8	2.49	2.71	1.36	1.36	1.36	1.36		
14	211263	129328	129328	195894	72402	9.7329	30	1.0956	9	8	0.86	1.07	1.13	1.25	1.13	1.25		

NOTES:
 Seismic Stresses: *Allowable stresses may be increased 1/3 above the values otherwise provided when produced by wind or seismic loading ...
 AISC MSC (Chap. A Sect. A5)
 In Zone 8, where the Upper Gusset Weldment bolts to the Flux Return center gap plate, A490 bolts were used because Kawasaki drilled and tapped 26 x 1" diameter holes in the gap plate because of confusion between SLAC and Kawasaki.

Table #3: Pin Sizing

Pin Properties Assume 4140 Normalized steel

Yield Strength: $S_y = 95000$ psi

Shear Strength: $S_{sy} = 54815$ psi

Ultimate Strength: $S_u = 148000$ psi

Pins

Zone	Seismic Shear Force (lb)	Static Shear Force (lb)	Pin Diam (mm)	Minimum Number of Pins Req..	Chosen Number of Pins	Seismic Safety Factor	Static Safety Factor
Variable	F_{SHEAR}	F_A	d	N_{MIN}	N	SF_{PINS}	SF_{PINS}
Equation				Equation #5		Equation #6	Equation #6
1	50837	46153	12	6	10	1.89	2.08
2	109984	69830	-	-	-	-	-
3	96575	89213	30	2	7	4.35	4.71
4	250747	208846	30	5	7	1.68	2.01
5	165865	151338	-	-	-	-	-
6	236890	194772	-	-	-	-	-
7	167596	27810	-	-	-	-	-
8	116658	106775	-	-	-	-	-
9	197070	139267	-	-	-	-	-
10	28319	10117	-	-	-	-	-
11	120000	120000	30	2	4	2.00	2.00
12	152000	152000	-	-	-	-	-
13	93783	87498	30	2	7	4.48	4.80
14	211263	195894	30	4	7	1.99	2.15

Table #4: Bolt and Pin Sizing Data						
ZONE #1	T-bar to Outer Ring Segments					
	Face	Direction	Combination 1	Combination 2	Combination 3	Combination 4
			Force (lb)	Force (lb)	Force (lb)	Force (lb)
	1	Shear	41247	38097	40926	38704
	1	Axial	1114	906	3005	1287
	2	Shear	45704	42892	48751	43510
	2	Axial	1106	791	3122	1465
	3	Shear	33343	30827	31255	29947
	3	Axial	1612	1491	3034	1588
	4	Shear	36646	34212	36777	33758
	4	Axial	371	121	189	339
	5	Shear	50837	48890	46515	46377
	5	Axial	1869	2389	5237	1739
	6	Shear	50167	47707	46791	46153
	6	Axial	1476	1744	4472	1472
MAX SHEAR FORCE =			50837 lb			
MAX AXIAL FORCE =			5237 lb			
STATIC SHEAR FORCE =			46153 lb			
STATIC AXIAL FORCE =			1739 lb			
ZONE #2	T-bar to Inner Ring					
	Face	Direction	Combination 1	Combination 2	Combination 3	Combination 4
			Force (lb)	Force (lb)	Force (lb)	Force (lb)
	1	Shear	58528	56660	46556	56466
	1	Axial	5346	7266	25836	6874
	2	Shear	13490	12961	24890	14899
	2	Axial	2037	2024	4908	2169
	3	Shear	6923	8212	18464	6648
	3	Axial	6156	6304	14242	6223
	4	Shear	71056	67536	109984	69830
	4	Axial	21180	14180	55380	25810
MAX SHEAR FORCE =			109984 lb			
MAX AXIAL FORCE =			55380 lb			
STATIC SHEAR FORCE =			69830 lb			
STATIC AXIAL FORCE =			25810 lb			
ZONE #3	Upper T-bar to Upper Gusset					
	Direction	Combination 1	Combination 2	Combination 3	Combination 4	
		Force (lb)	Force (lb)	Force (lb)	Force (lb)	
	x-direction	980	2452			
	y-direction	22988	-19640	23184	-17408	
	z-direction	93751	-89999	-78816	-87498	
MAX SHEAR FORCE =			96575 lb			
MAX AXIAL FORCE =			2452 lb			
STATIC SHEAR FORCE =			89213 lb			
STATIC AXIAL FORCE =			0 lb			
ZONE #4	Lower T-bar to Lower Gusset					
	Direction	Combination 1	Combination 2	Combination 3	Combination 4	
		Force (lb)	Force (lb)	Force (lb)	Force (lb)	
	x-direction	1540	3848			
	y-direction	74074	73072	129328	72402	
	z-direction	211228	-2022208	-214822	-195894	
MAX SHEAR FORCE =			250747 lb			

MAX AXIAL FORCE =		3848	lb			
STATIC SHEAR FORCE =		208846	lb			
STATIC AXIAL FORCE =		0	lb			
ZONE #5	Upper Gusset to Horse Collar					
TOP	Direction	Combination 1	Combination 2	Combination 3	Combination 4	
		Force (lb)	Force (lb)	Force (lb)	Force (lb)	
	x-direction	540	1350			
	y-direction	165860	-15714	-115304	-151338	
	z-direction	30862	-27248	-17910	-24840	
MAX SHEAR FORCE =		165865	lb			
MAX AXIAL FORCE =		30862	lb			
STATIC SHEAR FORCE =		151338	lb			
STATIC AXIAL FORCE =		24840	lb			
ZONE #6	Bottom Gusset to Horse Collar					
BOTOM	Direction	Combination 1	Combination 2	Combination 3	Combination 4	
		Force (lb)	Force (lb)	Force (lb)	Force (lb)	
	x-direction	936	2340			
	y-direction	213010	202066	236878	194772	
	z-direction	34606	-30862	-3410	-28368	
MAX SHEAR FORCE =		236890	lb			
MAX AXIAL FORCE =		34606	lb			
STATIC SHEAR FORCE =		194772	lb			
STATIC AXIAL FORCE =		28368	lb			
ZONE #7	Top Brace to Upper Gusset					
	Direction	Combination 1	Combination 2	Combination 3	Combination 4	
		Force (lb)	Force (lb)	Force (lb)	Force (lb)	
	x-direction	18424	24846	28446	16414	
	y-direction	3592	-6572	-7916	-4972	
	z-direction	78964	165164	37334	22450	
MAX SHEAR FORCE =		167596	lb			
MAX AXIAL FORCE =		-7916	lb			
STATIC SHEAR FORCE =		27810	lb			
STATIC AXIAL FORCE =		4972	lb			
ZONE #8	Chimney Flange to IFR Barrel					
	Direction	Combination 1	Combination 2	Combination 3	Combination 4	
		Force (lb)	Force (lb)	Force (lb)	Force (lb)	
	x-direction	78850	72958	69850	73056	
	y-direction	85976	77279	31625	77870	
	z-direction	43853	39874	25702	39305	
MAX SHEAR FORCE =		116658	lb			
MAX AXIAL FORCE =		43853	lb			
STATIC SHEAR FORCE =		106775	lb			
STATIC AXIAL FORCE =		39305	lb			
ZONE #9	Lower Gusset Flange to IFR Barrel					
	Direction	Combination 1	Combination 2	Combination 3	Combination 4	
		Force (lb)	Force (lb)	Force (lb)	Force (lb)	
	x-direction	17593	9310	29238	21092	
	y-direction	-146002	-137594	-194889	-137661	
	z-direction	26977	-16972	71234	48665	
MAX SHEAR FORCE =		197070	lb		Max Axial	
MAX AXIAL FORCE =		71234	lb		Force From	
STATIC SHEAR FORCE =		139267	lb		Hand Calc =	263500
STATIC AXIAL FORCE =		48665	lb			
ZONE #10	Top Brace to IFR Barrel					

	Direction	Combination 1	Combination 2	Combination 3	Combination 4
		Force (lb)	Force (lb)	Force (lb)	Force (lb)
	x-direction	-13654	-19195	-16319	-10117
	y-direction	-1126	-1221	-3592	-1105
	z-direction	-15117	-20496	-20821	-13023
MAX SHEAR FORCE =		28319 lb			
MAX AXIAL FORCE =		3592 lb			
STATIC SHEAR FORCE =		10117 lb			
STATIC AXIAL FORCE =		1105 lb			
ZONE #11	Upper Plug Stop to Inner Ring				
	Direction	Force (lb)			
	x-direction	0			
	y-direction	343			
	z-direction	120000			
MAX SHEAR FORCE =		120000 lb			
MAX AXIAL FORCE =		343 lb			
STATIC SHEAR FORCE =		120000 lb			
STATIC AXIAL FORCE =		0 lb			
ZONE #12	Lower Stationary Plug to Inner Ring				
	Direction	Force (lb)			
	x-direction	0			
	y-direction	94400			
	z-direction	152000			
MAX SHEAR FORCE =		152000 lb			
MAX AXIAL FORCE =		94400 lb			
STATIC SHEAR FORCE =		152000 lb			
STATIC AXIAL FORCE =		94400 lb			
ZONE #13	Upper T-bar to Gusset Bracket				
	Direction	Combination 1	Combination 2	Combination 3	Combination 4
		Force (lb)	Force (lb)	Force (lb)	Force (lb)
	x-direction	980	2452		
	y-direction	22988	-19640	23184	-17408
	z-direction	93751	-89999	-78816	-87498
MAX SHEAR FORCE =		93783 lb			
MAX AXIAL FORCE =		23184 lb			
STATIC SHEAR FORCE =		87498 lb			
STATIC AXIAL FORCE =		17408 lb			
ZONE #14	Lower T-bar to Gusset Bracket				
	Direction	Combination 1	Combination 2	Combination 3	Combination 4
		Force (lb)	Force (lb)	Force (lb)	Force (lb)
	x-direction	1540	3848		
	y-direction	74074	73072	129328	72402
	z-direction	211228	-2022208	-214822	-195894
MAX SHEAR FORCE =		211263 lb			
MAX AXIAL FORCE =		129328 lb			
STATIC SHEAR FORCE =		195894 lb			
STATIC AXIAL FORCE =		72402 lb			

Table #5: Internal Bolt Sizing

Bolt Properties: Assume A325 Bolts
 Source: *AISC Manual of Steel Construction, Ninth Edition*
 Yield Strength $F_y = 81000$ psi

Assume Class B Contact Surfaces
 Slip Coefficient $\mu_s = 0.5$
 Allowable Slip Load per Unit of Bolt Area = $F_v = 28000$ psi
 For Oversized Holes = $F_v = 24000$ psi
 Tighten bolts to: 70% of Yield Strength

Bolts sized to resist shear force with friction

Zone	Variable	Equation	Shear Force (lb)		Static Axial Force (lb)	Total Stress Area Req. (in ²)	Bolt Diam. (mm)	Area of Bolt (in ²)	Minimum Number of Bolts Req. N_{MIN}	Chosen Number of Bolts	Safety Factor (slippage)		Safety Factor (bolt yield)	
			F_{shear}	F_A							F_{shear}	F_A	SF _{slippage}	SF _{yield}
A	20139	33756	12864	27447	1.3057	24	0.7012	2	24	22.57	35.58	1.38	1.39	
B	34233	49889	34233	43197	2.0691	24	0.7012	3	60	33.69	33.79	1.40	1.40	
C	54797	68496	51211	59791	3.1057	24	0.7012	5	60	20.80	22.43	1.39	1.39	
D	73663	63990	11038	58626	3.7269	20	0.4869	8	12	1.78	12.20	1.20	1.21	
E	500	0	500	0	0.0176	4	0.0195	1	6	6.54	6.54	1.43	1.43	

ZONE DESCRIPTIONS

- A AF to HC
- B HC to CMR
- C CMR to SST
- D SST to TF
- E TF to CST I-beams

NOTES:

Seismic Stresses: *Allowable stresses may be increased 1/3 above the values otherwise provided when produced by wind or seismic loading
 AISC MSC (Chap. A Sect. A6)

Table #6: Internal Bolt Data

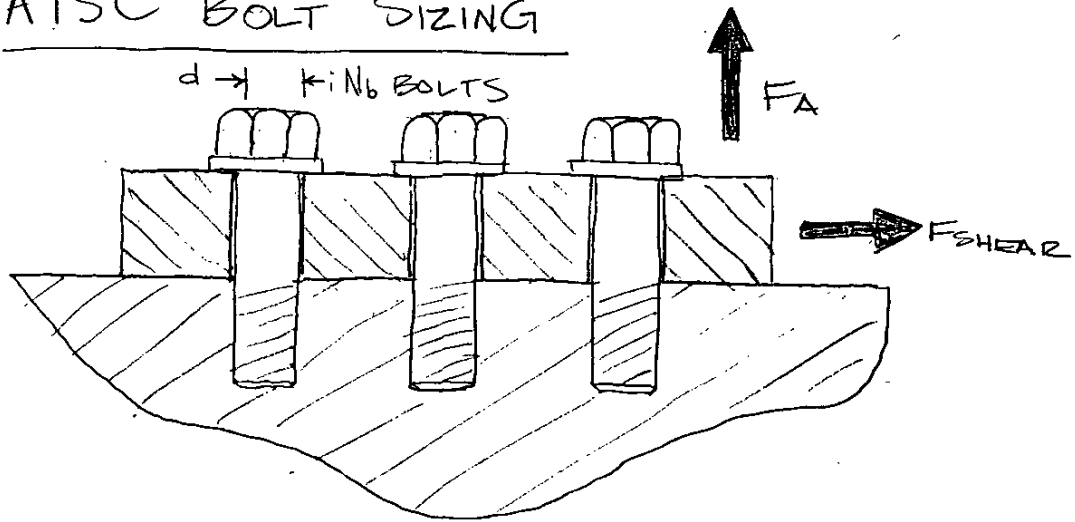
ZONE DESCRIPTIONS

- A AF to HC
- B HC to CMR
- C CMR to SST
- D SST to TF
- E TF to CST I-beams

DATA

ZONE	LOAD COMBINATION #1		LOAD COMBINATION #2		LOAD COMBINATION #3		LOAD COMBINATION #4		MAXIMUM VALUES	
	Shear Force (lb)	Axial Force (lb)	Shear Force (lb)	Axial Force (lb)	Shear Force (lb)	Axial Force (lb)	Shear Force (lb)	Axial Force (lb)	Shear Force (lb)	Axial Force (lb)
A	16273	27347	20139	33756	15619	26266	12864	27447	20139	33756
x-direction	-16259		-20074		-15180		-12588			
y-direction	674		1613		3675		2648			
B	35678	44347	29192	49989	31477	41465	34233	43197	35678	49989
x-direction	35640		29071		30823		34037			
y-direction	1649		2655		6385		3657			
C	54797	61942	47198	66496	47644	57377	51211	59791	54797	66496
x-direction	54741		47073		47255		51023			
y-direction	2482		3432		6078		4387			
D	10610	63990	10361	62106	30463	58405	11038	58626	73663	63990
x-direction	2213		-768		5102		3714			
y-direction	-10377		-10332		-30033		-10394			
E	11832	64913	12420	62848	32119	59828	11737	59828	500	0
x-direction	-1188		-4031		639		636			
y-direction	-11772		-11748		-32113		-11720			

AISC BOLT SIZING



AISC

F_u = ULTIMATE STRENGTH = $S_u t$
 F_y = YIELD STRENGTH = S_y

- IN ORDER TO PREVENT SLIPPAGE
ASSUME BOLTS MUST RESIST F_{SHEAR}
WITH FRICTION BETWEEN THE MEMBERS

$$F_{SHEAR} \leq M_s (F_i - F_A)$$

WHERE: $F_i = (0.70) S_y A_b N_b$ {ASTM MSC 5-274
TABLE 4}

$$F_i = \frac{F_{SHEAR}}{M_s} + F_A$$

$$(0.70) S_y A_b N_b = \frac{F_{SHEAR}}{M_s} + F_A$$

$$A_b N_b = A_T = \text{TOTAL STRESS AREA}$$

(AISC BOLT SIZING CONT.)

- SOLVING FOR TOTAL REQUIRED STRESS AREA, A_T :

$$A_T = \frac{\left(\frac{F_{\text{SHEAR}}}{M_S} \right) + F_A}{(0.70) S_y}$$

EQUATION #1
TOTAL
STRESS
AREA REQD.
PER BOLTING
ZONE

- USE A_T AND THE SELECTED SCREW DIAMETER, d , TO CALCULATE THE REQUIRED NUMBER OF BOLTS, N_b .

$$N_{\text{MIN}} = \frac{A_T}{\pi \frac{d^2}{4}}$$

EQUATION #2
NUMBER OF
BOLTS REQUIRED

- SAFETY FACTOR FOR BOLT SLIPPAGE:

THE FORCE ON A SLIP-CRITICAL JOINT SHALL NOT EXCEED THE ALLOWABLE RESISTANCE, P_s , OF THE CONNECTION:

$$P_s = F_s A_b N_b N_s \quad \left\{ \begin{array}{l} \text{AISC MSC 5-270} \\ 5(b) \end{array} \right.$$

F_s = ALLOWABLE SLIP LOAD PER UNIT OF BOLT AREA
 A_b = NOMINAL BODY AREA OF BOLT
 N_b = NUMBER OF BOLTS IN JOINT
 N_s = NUMBER OF SLIP PLANES

NOTE: FOR A325 BOLTS, ALLOWABLE SHEAR, F_v :

CLASS A (SLIP COEFFICIENT 0.33) , $F_v = 17000 \text{ psi}$ STANDARD SIZE HOLES

CLASS B (SLIP COEFFICIENT 0.50) , $F_v = 28000 \text{ psi}$

CLASS B OVERSIZED HOLES $F_v = 24000 \text{ psi}$ $\left\{ \begin{array}{l} \text{AISC MSC 5-271} \\ \text{TABLE 3} \end{array} \right.$

(AISC BOLT SIZING CONT.)

- COMBINED TENSION AND SHEAR IN SLIP-CRITICAL JOINTS:

REDUCTION FACTOR FOR F_v

$$F_s = F_v \left(1 - \frac{f_t A_b}{T_b} \right) \quad \left\{ \text{AISC MSC S-74} \right\}$$

f_t = AVE TENSILE STRESS IN BOLT DUE TO DIRECTLY APPLIED AXIAL LOAD TO BOLTS

T_b = PRETENSION LOAD OF THE BOLT

$$\left. \begin{aligned} T_b &= 0.70 S_y A_b \\ f_t &= \frac{F_A}{A_b N_b} \end{aligned} \right\} \Rightarrow F_s = F_v \left[1 - \frac{F_A}{N_b (0.70) S_y A_b} \right]$$

- SAFETY FACTOR FOR JOINT SLIPPAGE:

$$SF_{\text{BOLTS}} = \frac{F_s A_b N_b}{F_{\text{SHEAR}}}$$

$$\text{S.F.} = \frac{F_v \left[1 - \frac{F_A}{N_b (0.70) S_y A_b} \right] A_b N_b}{F_{\text{SHEAR}}}$$

EQUATION #3
SAFETY FACTOR
(SLIPPAGE)

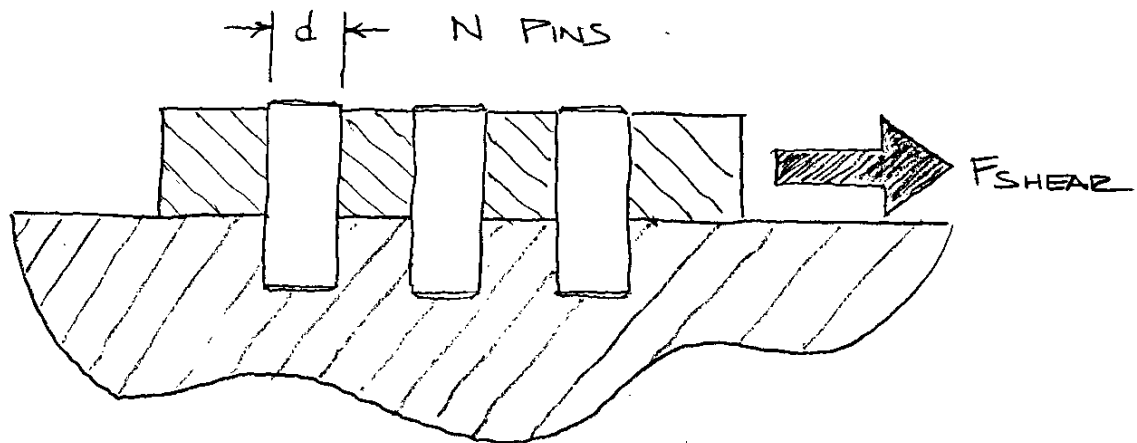
(AISC BOLT SIZING CONT.)

- SAFETY FACTOR FOR BOLT YIELDING
IN TENSION :

$$SF_{by} = \frac{S_y}{(0.70)S_y + \frac{F_A}{N_b A_b}}$$

EQUATION #4
SAFETY
FACTOR
(BOLT YIELD)

PIN SIZING



SHEAR STRENGTH OF THE PINS BASED ON
YIELD STRENGTH, S_y :

SHEAR STRENGTH = $S_{sy} = 0.577 S_y$
(BY DISTORTION ENERGY THEORY)

$$N_{MIN} = \frac{F_{SHEAR}}{S_{sy} \left(\frac{\pi d^2}{4} \right)}$$

EQUATION #5
NUMBER OF PINS
REQUIRED

SAFETY FACTOR FOR SLIPPAGE:

$$SF_{PINS} = \frac{S_{sy} N \left(\frac{\pi d^2}{4} \right)}{F_{SHEAR}}$$

EQUATION #6
SAFETY FACTOR
(SLIPPAGE)